

Memorandum

Date:	Friday, May 08,-2020	Revised Thursday, July 02, 2020
Project:	Lake Mendocino Foreca Assessment (FVA) Wate	st-Informed Reservoir Operations (FIRO) Full Viability er Control Plan (WCP) Alternative Analysis
To:	Jay Jasperse, PE and C	hris Delaney, PE
From:	Michael Konieczki, PE, I PE; and Melissa Larsen,	D.WRE (CA Lic. # 74357); Ric McCallan, PE; Megan Lionberger, PE
Subject:	WCP Alternative Analysi	s Results Metrics: Alternatives Comparison

Background

In 2014, Sonoma Water (SW) undertook a study to confirm the agency could manage Lake Mendocino storage more efficiently for authorized project purposes by integrating reservoir inflow forecasts explicitly in release schedule decision making. That study—which was referred to as the preliminary viability assessment (PVA)—confirmed SW could increase water supply benefit without adversely affecting the flood risk reduction capability if forecast-informed reservoir operation (FIRO) procedures were used. The U.S. Army Corps of Engineers (USACE), which is responsible for flood operation of Lake Mendocino, agreed with the finding and subsequently approved SW's request for a major deviation from the Lake Mendocino water control plan (WCP). This temporary deviation permitted SW greater flexibility in managing Lake Mendocino storage, pending additional investigation that would support incorporation of FIRO procedures in a formal revision of the WCP.

The PVA evaluated candidate FIRO strategies in a reconnaissance-level technical study, confirming viability of FIRO in concept. However, the PVA did not recommend a single specific strategy for integrating FIRO into a future WCP. That task is to be completed in a subsequent planning study—the Full Viability Assessment (FVA).

The objective of the FVA is to identify, through appropriate detailed technical analyses and other considerations, the best FIRO strategy for Lake Mendocino, along with the manner in which the strategy can be implemented in real-time operation by Sonoma Water (SW) and USACE, and enable the Water Control Plan (WCP) changes necessary to permit that change permanently. The FVA will also evaluate potential adaptive strategies that would allow operators to utilize new technology and improved forecast skill as it becomes available in the future.

The FVA is managed by the Lake Mendocino FIRO steering committee (SC), who identified technical studies consistent with USACE guidance needed for FIRO strategy analysis. The SC prepared a hydrologic engineering management plan (HEMP) that is "a technical outline of the hydrologic engineering studies necessary to formulate a solution to a water resources problem" (FIRO SC 2019). The objective of the HEMP is to identify and evaluate Lake Mendocino FIRO alternatives in a systematic, defendable, repeatable manner, providing information to the SC so it may identify the best FIRO strategy.

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The SC defined in the HEMP the set of 16 Metrics listed in Table 1 to evaluate the WCP alternatives consistently. In addition, the SC defined in the HEMP the 5 WCP alternatives listed in Table 2 to be evaluated for the FVA.

Metric	Metric Description
M1	Annual maximum flow frequency function at Hopland, Cloverdale, Healdsburg, and Guerneville
M2	Annual maximum pool elevation frequency function of Lake Mendocino
M3	Annual maximum pool elevation frequency function of Lake Sonoma
M4	Annual maximum Lake Mendocino total release frequency function
M5	Annual maximum Lake Sonoma total release function
M6	Annual maximum uncontrolled spill frequency function for Lake Mendocino
M7	Annual maximum uncontrolled spill frequency function for Lake Mendocino
M8	Expected annual inundation damage at critical Russian River locations
M9	Expected annual potential (statistical) loss of life due to floodplain inundation, critical Russian River locations
M10	Reliability of water supply delivery, as measured by annual exceedance frequency of Lake Mendocino May 10 reservoir storage levels
M11	The ability to meet instream flows to support threatened and endangered fish during the summer rearing season, as measured by the annual exceedance of the number of days June through September flows exceed 125 cfs
M12	The ability to meet instream flows to support fall spawning migration, as measured by the annual exceedance of the number of days October 15 to January 1 flows exceed 105 cfs
M13	Impacts to the Bushay Campground during the rec season (Memorial Day through Labor Day), as measured by the annual exceedance of the number of days that Lake Mendocino water-surface elevation exceeds 750 ft
M14	Impacts to power production of the CVD powerhouse
M15	Lake Mendocino bank protection, as measured by annual frequency of exceeding elevation 758.8 ft
M16	Impacts to hours of operation

Table 1. Summary of metrics identified in the HEMP



Table 2. Candidate FIRO alternatives to be evaluated

ID	WCP Alternative	Description
1	Existing (Baseline) Conditions	This is the baseline condition (existing WCP operations) against which performance of all alternatives will be measured. It includes the seasonal rule curve and release selection rules from the 1986 USACE WCM and 2003 update to the flood control diagram (FCD).
2	Ensemble Forecast Operations (EFO)	Operates without a traditional rule curve and uses the 15-day ensemble streamflow forecasts to identify required flood releases.
3	Hybrid (Major Deviation #1)	A combination of the Baseline WCP and the EFO. This WCP was used for Major Deviation Operations in WY19 and WY20.
4	Modified Hybrid	Identical to Hybrid but with a "corner cutting" strategy that allows for greater storage to begin February 15th to aid with spring refill.
5	5-Day Deterministic Forecast	Defines alternative guide curves with 11,000 AF encroachment space and 10,000 draft space above and below the Baseline guide curve. Uses 5-day deterministic inflow (and Hopland) forecasts to choose the guide curve and make release decisions.

Task

Operation of each Lake Mendocino WCP alternative was simulated using an HEC-ResSim model of the Russian River. The reservoir releases were then routed hydraulically using an HEC-RAS model. We were tasked with processing the HEC-ResSim and HEC-RAS model results to evaluate the metrics defined in the HEMP.

Action

To evaluate the WCP alternative we:

- 1. Coordinated with SW and USACE Hydrologic Engineering Center (HEC) staff to develop procedures for computing each metric. These procedures are detailed in 2 technical memoranda titled *Proposed Procedure for Consequence Analysis* and *Procedures for computation of non-consequence metrics* provided on 4/24/2020.
- 2. Coordinated with SW and USACE Hydrologic Engineering Center (HEC) staff to obtain HEC-ResSim and HEC-RAS model results.
- 3. Evaluated the 16 metrics for each WCP alternative using the agreed procedures and documented our findings in a series of technical memoranda.
- 4. Compared, by metric, the results of each WCP to the existing (baseline) conditions and documented out findings.



Study Area

Lake Mendocino, formed by the impoundment of the East Fork of the Russian River by the Coyote Valley Dam (CVD), is 3 miles east of the City of Ukiah, CA. Figure 1 shows its location. The 1,485-square mile Russian River watershed is a narrow valley between 2 adjacent northern coastal mountain ranges. The watershed is about 100 miles long and varies from 12 to 32 miles in width. Inflows to Lake Mendocino include runoff from an approximately 105-square mile drainage area and diversions from the Eel River to the East Fork of the Russian River above CVD through the Potter Valley Project. Some streamflows on the East Fork of the Russian River are diverted for irrigation purposes. Water from Lake Mendocino flows generally south down the East Fork Russian River until its confluence with the Russian River mainstem. Flow continues south near the towns of Hopland, Cloverdale, and Healdsburg. Just south of Healdsburg, Dry Creek flows into the Russian River from the west. The Russian River continues west past Guerneville to the Pacific Ocean.





Figure 1. Map of Russian River watershed, including Lake Mendocino



Findings

According to the HEMP, the efficacy of WCP alternatives must be evaluated using a set of measurable statistics that assess each alternative objectively. The SC defined in the HEMP a set of 16 Metrics as listed in Table 1 above. The following sections of this memo summarize the modeling results in terms of these metrics, and compare the performance of WCP alternatives to that of the existing (baseline) conditions.

Annual Maximum Flow-Frequency Functions (Metric 1)

M1 is calculated by post-processing HEC-ResSim output to determine the annual maximum flow for each water year in the period of record (POR) of 1/1/1985 through 9/30/2017 (water year [WY] 1985 to WY 2017), and the scaled 200-year and 500-year design floods (1986, 1995, 1997, and 2006). Table 3 through Table 10 and Figure 2 through Figure 5 show the annual exceedance probability (AEP) at Hopland, Cloverdale, Healdsburg, and Guerneville.

			Maximum Regulated Flow (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	11,143	10,677	11,143	10,972	10,677			
0.2	5	16,770	16,875	16,843	16,842	16,753			
0.1	10	25,502	25,100	25,099	25,098	25,128			
0.05	20	32,287	32,272	32,273	32,291	32,307			
0.02	50	35,118	34,991	35,043	35,123	35,153			
0.01	100	39,301	39,114	39,245	39,284	39,258			
0.005	200	43,129	42,886	43,091	43,091	43,014			
0.002	500	53,302	51,967	50,498	50,498	50,473			

Table 3. Annual maximum regulated flow frequency at Hopland

Table 4. Difference in annual maximum regulated flow frequency at Hopland

		Difference in Annual Maximum Regulated Flow Quantile (cfs) and [%]					
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	-466 [-4%]	0 [0%]	-171 [-2%]	-466 [-4%]		
0.2	5	105 [1%]	73 [0%]	72 [0%]	-17 [0%]		
0.1	10	-402 [-2%]	-403 [-2%]	-404 [-2%]	-374 [-1%]		
0.05	20	-15 [0%]	-14 [0%]	4 [0%]	20 [0%]		
0.02	50	-127 [0%]	-75 [0%]	5 [0%]	35 [0%]		
0.01	100	-187 [0%]	-56 [0%]	-17 [0%]	-43 [0%]		
0.005	200	-243 [-1%]	-38 [0%]	-38 [0%]	-115 [0%]		
0.002	500	-1335 [-3%]	-2804 [-5%]	-2804 [-5%]	-2829 [-5%]		

			Maximum Regulated Flow (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	15,936	15,299	15,456	15,314	15,237			
0.2	5	27,135	26,571	26,569	26,599	26,818			
0.1	10	37,795	37,786	37,771	37,946	37,797			
0.05	20	45,740	45,748	45,747	45,741	45,744			
0.02	50	51,693	51,658	51,738	51,731	51,726			
0.01	100	59,075	58,967	59,179	59,176	59,163			
0.005	200	65,831	65,656	65,989	65,990	65,968			
0.002	500	77,651	77,034	76,841	76,841	77,115			

Table 5. Annual maximum regulated flow frequency at Cloverdale

Table 6. Difference in annual maximum regulated flow frequency at Cloverdale

		Difference in Annual Maximum Regulated Flow Quantile (cfs) and [%]					
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	-637 [-4%]	-480 [-3%]	-622 [-4%]	-699 [-4%]		
0.2	5	-564 [-2%]	-566 [-2%]	-536 [-2%]	-317 [-1%]		
0.1	10	-9 [0%]	-24 [0%]	151 [0%]	2 [0%]		
0.05	20	8 [0%]	7 [0%]	1 [0%]	4 [0%]		
0.02	50	-35 [0%]	45 [0%]	38 [0%]	33 [0%]		
0.01	0.01 100 -108 [0		104 [0%]	101 [0%]	88 [0%]		
0.005	200 -175 [0%]		158 [0%]	159 [0%]	137 [0%]		
0.002 500		-617 [-1%]	-810 [-1%]	-810 [-1%]	-536 [-1%]		

			Maximum Regulated Flow (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	25,448	25,150	25,325	25,147	25,364			
0.2	5	43,105	42,271	42,528	42,704	42,861			
0.1	10	60,843	60,450	60,451	60,707	61,066			
0.05	20	75,949	75,980	75,981	75,973	75,985			
0.02	50	84,436	84,404	84,476	84,475	84,438			
0.01	100	97,646	97,550	97,743	97,751	97,652			
0.005	200	109,736	109,581	109,885	109,902	109,745			
0.002	500	127,529	127,222	127,099	127,099	127,159			

Table 7. Annual maximum regulated flow frequency at Healdsburg

Table 8. Difference in annual maximum regulated flow frequency at Healdsburg

		Difference in Annual Maximum Regulated Flow Quantile (cfs) and [%]					
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	-298 [-1%]	-123 [0%]	-301 [-1%]	-84 [0%]		
0.2	5	-834 [-2%]	-577 [-1%]	-401 [-1%]	-244 [-1%]		
0.1	10	-393 [-1%]	-392 [-1%]	-136 [0%]	223 [0%]		
0.05	20	31 [0%]	32 [0%]	24 [0%]	36 [0%]		
0.02	50	-32 [0%]	40 [0%]	39 [0%]	2 [0%]		
0.01	100	-96 [0%]	97 [0%]	105 [0%]	6 [0%]		
0.005	200	-155 [0%]	149 [0%]	166 [0%]	9 [0%]		
0.002	500	-307 [0%]	-430 [0%]	-430 [0%]	-370 [0%]		

			Maximum Regulated Flow (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	39,946	39,535	39,525	39,539	39,849			
0.2	5	61,252	61,061	61,130	61,164	61,199			
0.1	10	85,090	84,823	84,824	84,821	85,212			
0.05	20	100,176	99,785	99,786	100,143	100,291			
0.02	50	116,759	116,700	116,767	116,772	116,733			
0.01	100	135,402	135,245	135,426	135,440	135,332			
0.005	200	152,464	152,218	152,503	152,524	152,354			
0.002	500	179,745	179,248	179,063	179,061	179,162			

Table 9. Annual maximum regulated flow frequency at Guerneville

Table 10. Difference in annual maximum regulated flow frequency at Guerneville

		Difference in Annual Maximum Regulated Flow Quantile (cfs) and [%]					
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	-411 [-1%]	-421 [-1%]	-407 [-1%]	-97 [0%]		
0.2	5	-191 [0%]	-122 [0%]	-88 [0%]	-53 [0%]		
0.1	10	-267 [0%]	-266 [0%]	-269 [0%]	122 [0%]		
0.05	20	-391 [0%]	-390 [0%]	-33 [0%]	115 [0%]		
0.02	50	-59 [0%]	8 [0%]	13 [0%]	-26 [0%]		
0.01	100	-157 [0%]	24 [0%]	38 [0%]	-70 [0%]		
0.005	200	-246 [0%]	39 [0%]	60 [0%]	-110 [0%]		
0.002 500 -497 [09		-497 [0%]	-682 [0%]	-684 [0%]	-583 [0%]		





Figure 2. Annual maximum flow exceedance probability at Hopland





Figure 3. Annual maximum flow exceedance probability at Cloverdale





Figure 4. Annual maximum flow exceedance probability at Healdsburg





Figure 5. Annual maximum flow exceedance probability at Guerneville (Hacienda Bridge)



Lake Mendocino Annual Maximum Frequency Functions (Metrics 2, 4, and 6)

M2, M4, and M6 describe the maximum pool elevation, maximum total release, and uncontrolled spill frequency functions at Lake Mendocino. Table 11 through Table 20 summarize these functions and Figure 6 through Figure 8 show the functions graphically.

		Maximum Pool Elevation (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	742.77	753.81	746.77	748.90	748.32		
0.2	5	752.71	758.48	752.79	754.14	754.15		
0.1	10	754.52	759.40	755.20	755.32	755.95		
0.05	20	760.93	761.70	756.09	756.85	759.42		
0.02	50	766.14	763.39	759.13	758.95	761.65		
0.01	100	767.18	765.16	761.23	761.12	763.99		
0.005	200	768.13	766.77	763.15	763.10	766.13		
0.002	500	770.68	769.51	766.47	766.43	769.71		

Table 11. Annual maximum po	ol elevation in Lake Mendocino
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Table 1	2. Difference	in annual	maximum	pool elevation	on in La	ke Mendocino
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		Difference in Annual Pool Elevation Frequency Quantile (ft)							
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast				
0.5	2	11 [11%]	4 [4%]	6 [6%]	6 [5%]				
0.2	5	6 [5%]	0 [0%]	1 [1%]	1 [1%]				
0.1	10	5 [4%]	1 [1%]	1 [1%]	1 [1%]				
0.05	20	1 [1%]	-5 [-4%]	-4 [-3%]	-2 [-1%]				
0.02	50	-3 [-2%]	-7 [-6%]	-7 [-6%]	-4 [-4%]				
0.01	100	-2 [-2%]	-6 [-5%]	-6 [-5%]	-3 [-3%]				
0.005	200	-1 [-1%]	-5 [-4%]	-5 [-4%]	-2 [-2%]				
0.002	500	-1 [-1%]	-4 [-3%]	-4 [-3%]	-1 [-1%]				

		Maximum Storage (ac-ft)					
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast	
0.5	2	77,321	96,603	84,227	87,945	86,930	
0.2	5	94,648	104,954	94,798	97,182	97,203	
0.1	10	97,866	106,613	99,072	99,287	100,412	
0.05	20	109,378	110,794	100,664	102,022	106,640	
0.02	50	118,959	113,881	106,117	105,792	110,686	
0.01	100	120,888	117,130	109,927	109,720	114,974	
0.005	200	122,661	120,128	113,439	113,343	118,937	
0.002	500	127,482	125,269	119,556	119,491	125,643	

Table 13. Annual maximum storage in Lake Mendocino

Table 14. Difference in annual maximum storage in Lake Mendocino

		Difference in Annual Storage Frequency Quantile (ac-ft) and [%]							
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast				
0.5	2	19,281 [25%]	6,906 [9%]	10,623 [14%]	9,609 [12%]				
0.2	5	10,306 [11%]	149 [0%]	2,534 [3%]	2,555 [3%]				
0.1	10	8,746 [9%]	1,205 [1%]	1,421 [1%]	2,546 [3%]				
0.05	20	1,416 [1%]	-8,714 [-8%]	-7,356 [-7%]	-2,738 [-3%]				
0.02	50	-5,078 [-4%]	-12,841 [-11%]	-13,166 [-11%]	-8,273 [-7%]				
0.01	100	-3,757 [-3%]	-10,961 [-9%]	-11,167 [-9%]	-5,914 [-5%]				
0.005	200	-2,533 [-2%]	-9,222 [-8%]	-9,318 [-8%]	-3,724 [-3%]				
0.002	500	-2,213 [-2%]	-7,927 [-6%]	-7,992 [-6%]	-1,840 [-1%]				

		Maximum Total Release (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	3,850	3,278	3,550	3,400	3,924		
0.2	5	4,001	4,001	4,000	4,000	4,001		
0.1	10	4,001	4,002	4,001	4,001	4,002		
0.05	20	5,697	4,908	4,713	4,278	5,757		
0.02	50	6,686	6,056	5,304	5,304	6,020		
0.01	100	7,177	6,056	5,601	5,601	6,168		
0.005	200	7,626	6,056	5,873	5,873	6,304		
0.002	500	12,460	9,261	8,888	8,888	9,030		

Table 15. Annual maximum total release in Lake Mendocino

Table 16. Difference in annual maximum total release in Lake Mendocino

		Difference in Annual Total Release Frequency Quantile (ft) and [%]							
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast				
0.5	2	-572 [-15%]	-300 [-8%]	-450 [-12%]	74 [2%]				
0.2	5	0 [0%]	-1 [0%]	-1 [0%]	0 [0%]				
0.1	10	1 [0%]	0 [0%]	[0%]	1 [0%]				
0.05	20	-789 [-14%]	-984 [-17%]	-1419 [-25%]	60 [1%]				
0.02	50	-630 [-9%]	-1382 [-21%]	-1382 [-21%]	-666 [-10%]				
0.01	100	-1121 [-16%]	-1576 [-22%]	-1576 [-22%]	-1009 [-14%]				
0.005	200	-1570 [-21%]	-1753 [-23%]	-1753 [-23%]	-1322 [-17%]				
0.002	500	-3199 [-26%]	-3572 [-29%]	-3572 [-29%]	-3430 [-28%]				

		Uncontrolled Spill (cfs)					
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast	
0.5	2	0	0	0	0	0	
0.2	5	0	0	0	0	0	
0.1	10	0	0	0	0	0	
0.05	20	94	0	0	0	0	
0.02	50	1342	426	232	226	436	
0.01	100	2940	1133	618	602	1160	
0.005	200	4402	1781	971	946	1822	
0.002	500	7342	5260	3278	3231	5168	

Table 17. Annual maximum uncontrolled spill in Lake Mendocino

Table 18. Difference in annual uncontrolled spill frequency in Lake Mendocino

		Difference in Annual Uncontrolled Spill Frequency Quantile (ft) and [%]							
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast				
0.5	2	0 [0%]	0 [0%]	0 [0%]	0 [0%]				
0.2	5	0 [0%]	0 [0%]	0 [0%]	0 [0%]				
0.1	10	0 [0%]	0 [0%]	0 [0%]	0 [0%]				
0.05	20	-94 [-100%]	-94 [-100%]	-94 [-100%]	-94 [-100%]				
0.02	50	-916 [-68%]	-1110 [-83%]	-1116 [-83%]	-906 [-67%]				
0.01	100	-1807 [-61%]	-2322 [-79%]	-2338 [-80%]	-1780 [-61%]				
0.005	200	-2621 [-60%]	-3431 [-78%]	-3456 [-79%]	-2580 [-59%]				
0.002	500	-2082 [-28%]	-4064 [-55%]	-4111 [-56%]	-2174 [-30%]				



Lake Mendocino Full Viability Assessment Lake Mendocino Forecast-Informed Reservoir Operations (FIRO) Full Viability Assessment (FVA) Water Control Plan (WCP) Alternative Analysis: Alternatives Comparison

Table 19. Total hours of uncontrolled spill – 200-yr

	Hours of Uncontrolled Spill						
Scaled Event	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
1986	101	97	61	53	94		
1995	0	0	0	0	0		
1997	54	12	0	0	0		
2006	78	85	68	68	72		

Table 20. Total hours of uncontrolled spill – 500-yr

	Hours of Uncontrolled Spill								
Scaled Event	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast				
1986	108	111	95	92	102				
1995	7	22	0	0	18				
1997	65	64	0	0	53				
2006	82	96	79	79	79				





Figure 6. Annual maximum pool elevation-frequency in Lake Mendocino





Figure 7.Annual maximum total release-frequency in Lake Mendocino



Figure 8. Annual maximum uncontrolled spill-frequency in Lake Mendocino

Lake Sonoma Annual Maximum Flow Frequency Functions (Metrics 3, 5, and 7)

M3, M5, and M7 describe the maximum pool elevation, maximum total release, and uncontrolled spill frequency functions at Lake Sonoma. Table 21 through Table 30 summarize these functions and Figure 9 through Figure 11 show these functions graphically.

		Maximum Pool Elevation (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	454.55	454.61	454.61	454.61	454.61		
0.2	5	465.25	465.24	465.26	465.25	465.29		
0.1	10	477.51	477.21	479.05	479.05	479.07		
0.05	20	483.73	483.69	483.98	484.11	481.15		
0.02	50	485.80	485.91	486.50	486.92	485.63		
0.01	100	488.95	489.51	490.25	490.45	488.85		
0.005	200	491.84	492.80	493.69	493.69	491.81		
0.002	500	493.10	493.71	494.32	494.47	493.10		

Table 22. Difference in annual maximum pool elevation in Lake Sonoma

		Difference in Annual Pool Elevation Frequency Quantile (ft) and [%]						
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	0.06 [0%]	0.06 [0%]	0.06 [0%]	0.06 [0%]			
0.2	5	-0.01 [0%]	0.01 [0%]	0.00 [0%]	0.04 [0%]			
0.1	10	-0.30 [0%]	1.54 [1%]	1.54 1%]	1.56 [1%]			
0.05	20	-0.04 [0%]	0.25 [0%]	0.38 [0%]	-2.58 [-1%]			
0.02	50	0.11 [0%]	0.70 [0%]	1.12 [0%]	-0.17 [0%]			
0.01	100	0.56 [0%]	1.30 [0%]	1.50 [1%]	-0.10 [0%]			
0.005	200	0.96 [0%]	1.85 [1%]	1.85 [1%]	-0.03 [0%]			
0.002	500	0.61 [0%]	1.22 [0%]	1.37 [1%]	0.00 [0%]			

		Maximum Storage (ac-ft)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	254,370	254,532	254,532	254,532	254,532		
0.2	5	284,461	284,436	284,498	284,441	284,558		
0.1	10	321,805	320,849	326,723	326,723	326,792		
0.05	20	341,979	341,848	342,779	343,230	333,495		
0.02	50	348,849	349,227	351,221	352,632	348,280		
0.01	100	359,548	361,460	364,028	364,720	359,209		
0.005	200	369,538	372,909	376,027	376,027	369,416		
0.002	500	373,963	376,098	378,275	378,799	373,945		

Table 23. Annual maximum storage in Lake Sonoma

Table 24. Difference in annual maximum storage in Lake Sonoma

		Difference in Annual Storage Frequency Quantile (ac-ft) and [%]						
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	162 [0%]	162 [0%]	162 [0%]	162 [0%]			
0.2	5	-26 [0%]	36 [0%]	-20 [0%]	96 [0%]			
0.1	10	-955 [0%]	4,918 [2%]	4,918 [2%]	4,987 [2%]			
0.05	20	-131 [0%]	799 [0%]	1,250 [0%]	-8,485 [-2%]			
0.02	50	378 [0%]	2,372 [1%]	3,782 [1%]	-569 [0%]			
0.01	100	1,912 [1%]	4,479 [1%]	5,171 [1%]	-340 [0%]			
0.005	200	3,371 [1%]	6,489 [2%]	6,489 [2%]	-122 [0%]			
0.002	500	2,135 [1%]	4,312 [1%]	4,836 [1%]	-18 [0%]			

		Maximum Total Release (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	2,000	2,000	2,000	2,000	2,000		
0.2	5	4,385	4,385	4,385	4,385	4,385		
0.1	10	6,000	6,000	6,000	6,000	6,000		
0.05	20	6,000	6,000	6,000	6,000	6,000		
0.02	50	6,004	6,004	6,004	6,004	6,004		
0.01	100	6,010	6,010	6,010	6,010	6,010		
0.005	200	6,015	6,015	6,015	6,015	6,015		
0.002	500	6,015	6,015	6,015	6,015	6,015		

Table 25. Annual maximum total release in Lake Sonoma

Table 26. Difference in annual maximum total release in Lake Sonoma

		Difference in Annual Total Release Frequency Quantile (ft) and [%]						
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.2	5	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.1	10	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.05	20	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.02	50	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.01	100	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.005	200	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.002	500	0 [0%]	0 [0%]	0 [0%]	0 [0%]			

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		Uncontrolled Spill (cfs)						
Annual Exceedance Probability	1/AEP	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.5	2	0	0	0	0	0		
0.2	5	0	0	0	0	0		
0.1	10	0	0	0	0	0		
0.05	20	0	0	0	0	0		
0.02	50	299	306	302	302	308		
0.01	100	796	813	804	804	818		
0.005	200	1,251	1,277	1,263	1,263	1,285		
0.002	500	1,335	1,360	1,339	1,339	1,345		

Table 27. Annual maximum uncontrolled spill in Lake Sonoma

Table 28. Difference in annual uncontrolled spill frequency in Lake Sonoma

		Difference in Annual Uncontrolled Spill Frequency Quantile (ft) and [%]						
Annual Exceedance Probability	1/AEP	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.5	2	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.2	5	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.1	10	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.05	20	0 [0%]	0 [0%]	0 [0%]	0 [0%]			
0.02	50	7 [2%]	3 [1%]	3 [1%]	9 [3%]			
0.01	100	17 [2%]	8 [1%]	8 [1%]	22 [3%]			
0.005	200	26 [2%]	12 [1%]	12 [1%]	34 [3%]			
0.002	500	25 [2%]	4 [0%]	4 [0%]	10 [1%]			



	Hours of Uncontrolled Spill						
Scaled Event	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
1986	114	114	114	114	114		
1995	0	0	0	0	0		
1997	0	0	0	0	0		
2006	39	41	40	40	41		

Table 29. Total hours of uncontrolled spill – 200 year event

Table 30. Total hours of uncontrolled spill – 500 year event

	Hours of Uncontrolled Spill							
Scaled Event	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
1986	115	115	115	115	115			
1995	0	0	0	0	0			
1997	0	0	0	0	0			
2006	44	46	44	44	44			





Figure 9. Annual maximum pool elevation-frequency in Lake Sonoma





Figure 10. Annual maximum total release-frequency in Lake Sonoma



Figure 11. Annual maximum uncontrolled spill-frequency in Lake Sonoma



Expected Annual Damage (Metric 8)

Expected annual damage (EAD) is computed using an event-based approach that combines the hydrology from the historical period of record (POR) of Water Years (WY) 1986-2017 with design events representing hypothetical rare events not observed in the POR. The procedure for computing EAD is detailed in technical memorandum *Proposed Procedure for Consequence Analysis* provided 4/2/2020. Table 31 summarizes the EAD results by damage location.

	EAD (\$1,000) by WCP Alternative							
Location	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
Hopland	104.09	101.10	98.50	100.60	103.68			
Cloverdale	703.02	719.32	705.61	705.59	706.39			
Geyserville	191.71	185.16	189.74	189.74	189.40			
Healdsburg	542.20	532.17	533.05	535.03	540.82			
Dry Creek	2.63	2.66	2.69	2.68	2.68			
Windsor	265.56	259.56	258.48	258.48	260.23			
Santa Rosa	1,121.14	1,119.92	1,104.01	1,100.50	1,122.80			
Green Valley Creek	648.73	631.87	615.95	617.86	628.51			
Guerneville	11,282.16	11,207.26	11,065.81	11,049.95	11,274.18			
Monte Rio	369.84	366.74	364.47	363.75	370.06			
Total EAD	15,231.08	15,125.73	14,938.30	14,924.16	15,198.73			

Table 31. Expected annual damage



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Expected Annual Loss of Life (Metric 9)

Expected annual potential (statistical) loss of life (EALL) is computed using an event-based approach that combines the hydrology from the historical period of record (POR) of Water Years (WY) 1986-2017 with design events representing hypothetical rare events not observed in the POR. The procedure for computing EALL is detailed in the technical memorandum *Proposed Procedure for Consequence Analysis* provided 4/2/2020. Computation of EALL is dependent on assumptions of population demographics, warning times, evacuation routes, and so on, some of which are interrelated with the forecast and decision horizon of a specific WCP. Here, we report the expected annual population exposed to flooding (EAP) as analog for EALL. EAP is a function of floodplain hydraulics and population location (i.e. structure inventory geodata) and therefore is a direct measure of WCP performance. Table 32 summarizes the EAP results by damage location.

	EAP (persons) by WCP alternative						
Location	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
Hopland	15.3	14.7	14.6	14.7	14.9		
Cloverdale	42.8	42.7	42.4	42.4	42.6		
Geyserville	10.9	10.5	10.8	10.8	10.8		
Healdsburg	48.4	48.2	48.3	48.3	48.5		
Dry Creek	0.4	0.4	0.4	0.4	0.4		
Windsor	39.9	39.9	39.9	40.0	40.2		
Santa Rosa	101.5	100.8	99.2	99.1	101.5		
Green Valley Creek	2.6	2.5	2.5	2.5	2.5		
Guerneville	697.0	688.1	683.2	683.2	690.3		
Monte Rio	21.4	21.3	20.9	20.8	21.2		
Total EAP	980.2	969.1	962.2	962.2	972.9		

Table 32. Annual population exposed to flooding



Reliability of Water Supply Delivery (Metric 10)

The reliability of water supply delivery is represented by the May 10 storage in Lake Mendocino. The modeling results are extracted from HEC-ResSim output for each year in the POR of 1/1/1985 through 9/30/2017 (water year [WY] 1985 to WY 2017). Figure 12 shows the annual exceedance probability of Lake Mendocino storage on May 10 as a representation of annual water supply availability.

	Annual Exceedance Probability of Lake Mendocino Storage on May 10 th (ac-ft)				
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast
0.1	102,513	107,321	106,797	107,871	107,733
0.2	96,964	105,300	102,941	104,237	104,912
0.3	88,742	103,300	96,048	99,504	99,638
0.4	84,255	101,664	94,095	98,133	95,306
0.5	76,794	97,825	88,048	91,938	90,772
0.6	70,853	96,007	82,071	86,821	85,875
0.7	69,703	92,894	79,678	84,831	83,728
0.8	63,867	88,144	74,390	77,368	76,407
0.9	60,279	76,543	71,820	74,481	73,864

Table 33. Annual exceedance probability of Lake Mendocino storage on May 10th





Figure 12. Annual exceedance probability of Lake Mendocino storage on May 10th

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Figure 13. Difference in Lake Mendocino storage on May 10th in comparison to Baseline



Figure 14. Lake Mendocino storage on May 10th

FJS


The Ability to Meet Instream Flows to Support Threatened and Endangered Fish during the Summer Rearing Season (Metric 11)

M11 evaluates the ability of the alternatives to meet environmental flow targets for critical life-stage periods for anadromous fish in the reach below Lake Mendocino to the Cloverdale gage. Specifically, this metric represents the percent of days per summer rearing season in which flows exceed a target threshold established by the 2008 Biological Opinion in the Upper Russian River, 125 cfs. Table 34 through Table 36 and Figure 15 through Figure 17 present the percent of days per season in which flows exceed 125 cfs at critical downstream gage locations.

	East-West Junction					
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast	
0.1	1	1	1	1	1	
0.2	1	1	1 1 1		1	
0.3	1	1	1	1	1	
0.4	1	1	1	1	1	
0.5	1	1	1	1	1	
0.6	1	1	1	1	1	
0.7	1	1	1	1	1	
0.8	0.98	1	0.98	0.98	0.98	
0.9	0.91	0.97	0.93	0.93	0.93	

Table 34. Percent of days per season, June through September, in which flows exceed125 cfs at East-West Junction

Table 35. Percent of days per season, June through September, in which flows exceed 125 cfs at Hopland

		Hopland						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.1	1	1	1	1	1			
0.2	1	1	1 1		1			
0.3	1	1	1	1	1			
0.4	1	1	1	1	1			
0.5	1	1	1	1	1			
0.6	1	1	1	1	1			
0.7	0.61	1	1	1	1			
0.8	0.28	1	0.34	0.35	0.34			
0.9	0.14	0.31	0.22	0.22	0.22			



Table 36. Percent of days per season, June through September, in which flows exceed125 cfs at Cloverdale

	Cloverdale						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	1	1	1	1	1		
0.2	1	1	1	1	1		
0.3	1	1	1	1	1		
0.4	1	1	1	1	1		
0.5	1	1	1	1	1		
0.6	1	1	1 1 1		1		
0.7	0.69	1	1 1		1		
0.8	0.45	1	0.48	0.48	0.49		
0.9	0.41	0.46	0.42	0.43	0.42		

FSS



Figure 15. Percent of days per season, June through September, in which flows exceed 125 cfs at East-West Junction



Figure 16. Percent of days per season, June through September, in which flows exceed 125 cfs at Hopland

FSS



Figure 17. Percent of days per season, June through September, in which flows exceed 125 cfs at Cloverdale

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FX

Lake Mendocino Full Viability Assessment

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Figure 18. Difference in days per season, June through September, in which flows exceed 125 cfs in comparison to Baseline at East-West Junction



Figure 19. Difference in days per season, June through September, in which flows exceed 125 cfs in comparison to baseline at Hopland

FX



Figure 20. Difference in days per season, June through September, in which flows exceed 125 cfs in comparison to baseline at Cloverdale



Figure 21. Number of days per season, June through September, in which flows exceed 125 cfs at East-West Junction – box-and-whisker plot



Figure 22. Number of days per season, June through September, in which flows exceed 125 cfs at Hopland – box-and-whisker plot



Figure 23. Number of days per season, June through September, in which flows exceed 125 cfs at Cloverdale – box-and-whisker plot



The Ability to Meet Instream Flows to Support Fall Spawning Migration (Metric 12)

M12 evaluates the ability of the alternatives to meet environmental flow targets for critical life-stage periods for anadromous fish. This metric represents the percent of days per fall spawning season in which flows exceed a target threshold established by the 2008 Biological Opinion in the Upper Russian River, which is 105 cfs, except at the Hacienda Bridge gage downstream of Guerneville where it is 135 cfs. Table 37 though Table 41 and Figure 24 through Figure 28 present the percent of days per season in which flows exceed instream flow requirements.

	Flows Exceeding 105 cfs at East-West Junction							
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.1	1	1	1	1	1			
0.2	1	1	1	1	1			
0.3	1	1	1	1	1			
0.4	1	1	1	1	1			
0.5	1	1	1	1	1			
0.6	0.87	1	1	1	1			
0.7	0.69	1	0.98	1	0.98			
0.8	0.51	0.91	0.63	0.63	0.63			
0.9	0.42	0.57	0.47	0.47	0.53			

Table 37. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at East-West Junction

Table 38. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at Hopland

	Flows Exceeding 105 cfs at Hopland						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	1	1	1	1	1		
0.2	1	1	1	1	1		
0.3	1	1	1	1	1		
0.4	1	1	1	1	1		
0.5	1	1	1	1	1		
0.6	0.92	1	1	1	1		
0.7	0.72	1	0.97 1.00		0.97		
0.8	0.54	0.98	0.70	0.70	0.70		
0.9	0.39	0.68	0.42	0.42	0.45		



Table 39. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at Cloverdale

	Flows Exceeding 105 cfs at Cloverdale						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	1	1	1	1	1		
0.2	1	1	1	1	1		
0.3	1	1	1	1	1		
0.4	1	1	1	1	1		
0.5	1	1	1	1	1		
0.6	1	1	1	1	1		
0.7	0.86	1	0.99	1	1		
0.8	0.79	0.99	0.84	0.86	0.85		
0.9	0.67	0.85	0.70	0.70	0.69		

Table 40. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at Healdsburg

	Flows Exceeding 105 cfs at Healdsburg						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	1	1	1	1	1		
0.2	1	1	1 1		1		
0.3	1	1	1	1	1		
0.4	1	1	1	1	1		
0.5	1	1	1	1	1		
0.6	1	1	1	1	1		
0.7	0.79	1	1	1	1		
0.8	0.72	0.99	0.76	0.76	0.78		
0.9	0.56	0.76	0.56	0.56	0.65		



Table 41. Percent of days per season, October 12 through January 1, in which flows exceed 135 cfs at Guerneville (Hacienda Bridge)

	Flows Exceeding 135 cfs at Guerneville (Hacienda Bridge)						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	1	1	1	1	1		
0.2	1	1	1	1	1		
0.3	1	1	1	1	1		
0.4	1	1	1	1	1		
0.5	1	1	1	1	1		
0.6	1	1	1	1	1		
0.7	1	1	1	1	1		
0.8	0.97	1	0.97	0.97	0.97		
0.9	0.74	0.74	0.74	0.74	0.74		



Figure 24. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at East-West Junction



Figure 25. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at Hopland



Figure 26. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at Cloverdale



Figure 27. Percent of days per season, October 12 through January 1, in which flows exceed 105 cfs at Healdsburg



Figure 28. Percent of days per season, October 12 through January 1, in which flows exceed 135 cfs at Guerneville (Hacienda Bridge)



Figure 29. Difference in number of days per season, October 12 through January 1, in which flows exceed 105 cfs in comparison to Baseline at East-West Junction



Figure 30. Difference in number of days per season, October 12 through January 1, in which flows exceed 105 cfs in comparison to baseline at Hopland

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Figure 31. Difference in number of days per season, October 12 through January 1, in which flows exceed 105 cfs in comparison to baseline at Cloverdale



Figure 32. Difference in number of days per season, October 12 through January 1, in which flows exceed 105 cfs in comparison to baseline at Healdsburg

Difference in Number of Days per Season above 135 cfs from Baseline 0

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Figure 33. Difference in number of days per season, October 12 through January 1, in which flows exceed 135 cfs in comparison to baseline at Guerneville (Hacienda Bridge)

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Figure 34. Number of days per season, October 12 through January 1, in which flows exceed 105 cfs at East-West Junction – box-and-whisker plot



Figure 35. Number of days per season, October 12 through January 1, in which flows exceed 105 cfs at Hopland – box-and-whisker plot



Figure 36. Number of days per season, October 12 through January 1, in which flows exceed 105 cfs at Cloverdale – box-and-whisker plot



Figure 37. Number of days per season, October 12 through January 1, in which flows exceed 105 cfs at Healdsburg – box-and-whisker plot



Figure 38. Number of days per season, October 12 through January 1, in which flows exceed 135 cfs at Guerneville – box-and-whisker plot

Impacts to the Bushay Campground (Metric 13)

M13, presented in assesses the impacts to Bushay Campground. It is the inverse calculation of M11 and M12. It is the number of days a critical Lake Mendocino water-surface elevation exceeds a critical threshold during the recreation season: Memorial Day weekend through Labor Day weekend. Table 42 and Table 43 show the percent of days during which access is limited. Table 42 presents the data for a season while Table 43 presents the data for the whole calendar year. Figure 39, Figure 40 and Figure 41 show the percent of days and number of days in which the campground has limited access.

	Percent of Days per Season						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	1	1	1	1	1		
0.2	0.68	1	0.95	1	1		
0.3	0.25	0.92	0.68	0.86	0.85		
0.4	0	0.67	0.33	0.43	0.38		
0.5	0	0.54	0	0.29	0.03		
0.6	0	0.41	0	0	0		
0.7	0	0.21	0	0	0		
0.8	0	0	0	0	0		
0.9	0	0	0	0	0		

Table 42. Percent of days per recreation season during which access to Bushay Campground is limited (pool elevation 750.0 feet is exceeded)

Table 43. Percent of days per year during which access to Bushay Campground is limited (pool elevation 750.0 feet is exceeded)

	Percent of Days per Year						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	0.28	0.28	0.28	0.28	0.28		
0.2	0.19	0.28	0.26	0.28	0.28		
0.3	0.07	0.25	0.19	0.24	0.23		
0.4	0	0.18	0.09	0.12	0.11		
0.5	0	0.15	0	0.08	0.01		
0.6	0	0.11	0	0	0		
0.7	0	0.06	0	0	0		
0.8	0	0	0	0	0		
0.9	0	0	0	0	0		

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Figure 39. Percent of days per recreation season and per year during which access to Bushay Campground is limited (pool elevation 750.0 feet is exceeded)



Figure 40. Difference in number of days per recreation season during which access to Bushay Campground is limited (pool elevation 750.0 feet is exceeded) in comparison to Baseline

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Figure 41. Number of days per recreation season during which access to Bushay Campground is limited (pool elevation 750.0 feet is exceeded) – box-and-whisker plot



Impacts to Power Production of the CVD Powerhouse (Metric 14)

M14 evaluates the impact of the WCP on power production. To calculate M14, the HEC-ResSim results were post-processed to compute a timeseries of power production for the 33-year period of record. These results were used to compute statistics and exceedance plots on annual power production during the calendar year, December through March, and April through November. The power production exceedance values for Lake Mendocino are shown in Table 44 through Table 46 and the exceedance plots are shown in Figure 42 through Figure 50.

	Annual Power Production (MWh)							
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast			
0.1	10,972	9,583	11,243	10,608	10,972			
0.2	10,292	9,353	10,523	10,314	10,425			
0.3	9,774	8,556	10,172	9,935	10,129			
0.4	9,301	7,965	9,595	9,403	9,747			
0.5	8,773	7,655	9,206	9,002	9,135			
0.6	8,682	7,558	8,933	8,793	8,725			
0.7	8,278	7,371	8,230	8,377	8,129			
0.8	7,759	7,127	7,173	7,266	7,646			
0.9	6,195	5,824	5,968	6,077	6,218			

Table 44. Annual	(calendar	year)	power	production	exceedance	values
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Table 45. December through March power production exceedance values

	Power Production Dec-Mar (MWh)						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	4,480	3,222	4,650	4,634	5,163		
0.2	3,966	2,994	4,294	4,175	4,508		
0.3	3,765	2,729	3,867	3,602	4,117		
0.4	3,514	2,621	3,398	3,222	3,509		
0.5	3,310	1,954	3,246	3,062	3,216		
0.6	2,957	1,840	2,604	2,563	2,774		
0.7	1,910	1,604	2,071	2,161	1,993		
0.8	1,615	1,436	1,635	1,691	1,504		
0.9	1,131	1,234	1,284	1,363	1,447		



	Power Production Apr-Nov (MWh)						
Percent Exceedance	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast		
0.1	7,320	7,836	7,636	7,742	7,611		
0.2	7,200	7,605	7,428	7,563	7,503		
0.3	6,932	7,138	7,149	7,318	7,226		
0.4	6,710	5,922	6,953	6,406	6,313		
0.5	5,669	5,618	6,278	5,650	5,744		
0.6	5,316	5,219	5,531	5,556	5,558		
0.7	5,097	4,781	4,885	4,957	4,967		
0.8	4,858	4,298	4,710	4,739	4,718		
0.9	4,365	3,991	4,439	4,411	4,360		

Table 46. April through November power production exceedance values





Figure 42. Annual (calendar year) power production exceedance plot
FSS



Figure 43. December through March power production exceedance plot





Figure 44. April through November power production exceedance plot



Figure 45. Difference in annual power production in comparison to Baseline

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Figure 46. Difference in December through March power production in comparison with baseline



Figure 47. Difference in April through November power production in comparison to Baseline



Figure 48. Annual (calendar year) power production – box-and-whisker plot



Figure 49. December through March power production – box-and-whisker plot



Figure 50. April through November power production – box-and-whisker plot



Lake Mendocino Bank Protection (Metric 15)

M15, Lake Mendocino bank protection, is measured by the frequency that Lake Mendocino water surface elevation exceeds 758.8 ft on an annual basis (Table 47). Above this elevation, riprap shore protection is limited. For existing conditions, the pool elevation will exceed 758.8 ft at an AEP = 0.062 (approximately a 16-year return period). The number of days per season in which an elevation of 758.8 ft is exceeded is presented in Table 48 and Figure 51.

WCP	AEP at 758.8 ft	1/AEP
Baseline	0.062	16
EFO	0.174	6
Hybrid	0.022	45
Modified Hybrid	0.021	48
5-Day Deterministic Forecast	0.062	16

Table 47. Annual frequency of exceeding pool elevation 758.8 ft in Lake Mendocino

Table 48. Annual number of days per water year exceeding pool elevation 758.8 ft in Lake Mendocino

	Annual Number of Days per water year exceeding 758.8 ft in Lake Mendocino								
Percentile	Baseline	EFO	Hybrid	Modified Hybrid	5-Day Deterministic Forecast				
0.1	56	75	75	80	80				
0.2	17	68	52	67	67				
0.3	0	34	0	0	0				
0.4	0	17	0	0	0				
0.5	0	0	0	0	0				
0.6	0	0	0	0	0				
0.7	0	0	0	0	0				
0.8	0	0	0	0	0				
0.9	0	0	0	0	0				

FX



Figure 51. Number of days per water year exceeding pool elevation 758.8 ft in Lake Mendocino



Figure 52. Difference in Lake Mendocino pool elevation in comparison to Baseline



Figure 53. Difference in number of days per water year above pool elevation 758.8 ft in comparison to Baseline



Figure 54. Lake Mendocino pool elevation – box-and-whisker plot



Figure 55. Number of days per water year above pool elevation 758.8 ft



Impact to Hours of Operation (Metric 16)

The method in which HEC-ResSim makes flood releases from Lake Mendocino differs from the way that Sonoma Water releases flood waters in reality. There are a number of inconsistencies regarding operations assumptions in HEC-ResSim, including:

- The model contains flow ratings for both the gate and powerhouse, but since only one can be used at a time, the model's total capacity for the controlled outlets is generally too high.
- The model first allocates flow through powerhouse and uses the gate secondly. The model allows flow through both outlets simultaneously.
- Above 755 ft in elevation, the model constrains all controlled release to the gate. No powerhouse releases are allowed.

The total release simulated by the model is adequate, on a mass balance basis; however, the flow split between the two controlled outlets is not. To compensate for HEC-ResSim model shortcomings, HDR reallocates the releases based on the combined powerhouse and gate release output from the HEC-ResSim model assuming the following:

- Controlled releases are made through a single outlet, either the powerhouse or the controlled spillway, but not both simultaneously.
- Controlled releases less than 3,000 cfs are through the powerhouse, regardless of watersurface elevation.
- Controlled releases greater than 3,000 cfs are through the gate.

Each time flow through the gate goes up or down, it is counted as gate change, or a change operation. M16 quantifies the number of hourly gate changes for the period of record. The impacts to hours of operation, measured as the cumulative number of hourly gate changes throughout the period of record, is shown in Figure 56.



Figure 56. Count of cumulative gate changes



Figure 57. Difference in annual count of hourly gate changes in comparison with Baseline



Figure 58. Annual count of hourly gage changes

Key Findings

After reviewing the analysis results for these 16 metrics, we identified 8 key findings:

- The annual frequency and magnitude of uncontrolled spills at Lake Mendocino are reduced for all FIRO WCPs as shown in Figure 8 and Table 17.
- The annual flow frequency quantiles at Hopland for events less frequent than the p=0.5 (1/2yr) event are generally the same (within 1% of baseline) and decrease by up to 5% from baseline for the p=0.002 (1/500-yr) event for all FIRO WCPs as shown in Figure 2 and Table 4.
- The total EAD and EAP values for the Russian River are generally the same (within 1% of baseline) and may decrease slightly for all FIRO WCPs as shown in Table 31 and Table 32 However, we did find that EAD values for all WCPs along the reach from Hopland to Cloverdale showed slight (within 2%) increases from baseline. This increase in total EAD is because of increased damages to non-residential structures for specific events simulated. EAP values for this reach are generally the same (within 1%). Similarly, the reach including Dry Creek shows slightly (within 4%) increased EAD values for this reason. In addition, the 5-day Deterministic Forecast alternative shows slight (less than 1%) increases in total EAD for the reaches of Santa Rosa and Monte Rio for the same reason.
- The water supply reliability—as measured by the median (50th percentile exceedance) of May 10 storage—increases for all FIRO WCPs as shown in Figure 12.
- The ability to meet instream flows for rearing or spawning habitat generally increases for all for all FIRO WCPs as exemplified in Figure 15 through Figure 38.
- All FIRO WCPs would negatively impact the ability to access Bushay Campground during the recreation season (Memorial Day to Labor Day) as shown in Figure 41.
- City of Ukiah hydropower generation increases slightly (~4%) for the Hybrid, Modified Hybrid, and 5-day Deterministic Forecast WCPs, and decreases by 13% for the EFO WCP as shown in Figure 48.
- There are no impacts on Lake Sonoma operations as shown in Figure 10.

Performance summary

We ranked the performance of each WCP for the 16 metrics to understand better the information within the evaluated metrics. For several metrics, the process was complicated by multiple locations and WCP performance within the most important range of the frequency distributions. Table 49 lists the criteria we used to rank the metrics. Additionally, differences within 1% of each other were considered to be generally the same performance and thus given the same rank. In addition, we grouped the metrics into the following 4 categories (as listed in column 3 of Table 49):

- Flood risk management
- Water supply and environmental outcomes
- Recreation, power production, and staffing impacts



• Impacts to Lake Sonoma operations

Table 50 through Table 53 list the categorized rankings of each WCP by metric; color-coding them to display a "heat map" of performance.

Table 49.	Summary	of	methods	to	rank	WCP	by	metric
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Metric	Ranking method	Category
M1	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$), (2) the expected value [$p=0.5$], and (3) AEP of the flood flow at each location averaged across all locations.	Flood risk management
M2	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$) and (2) the expected value [$p=0.5$].	Flood risk management
M3	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$) and (2) the expected value [$p=0.5$].	Impacts to Lake Sonoma operations
M4	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$) and (2) the expected value [$p=0.5$].	Flood risk management
M5	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$) and (2) the expected value [$p=0.5$].	Impacts to Lake Sonoma operations
M6	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$) and (2) the expected value [$p=0.5$].	Flood risk management
M7	Average ranking of (1) flow quantiles in the tail of the distribution [$p=0.01$ through $p=0.002$) and (2) the expected value [$p=0.5$].	Impacts to Lake Sonoma operations
M8	Rank of difference in total EAD from baseline.	Flood risk management
M9	Rank of difference in total EAP from baseline.	Flood risk management
M10	Ranking of storage median (50%) exceedance values.	Water supply and environmental outcomes
M11	Ranking of 75% of period exceedance values at each location averaged across all locations.	Water supply and environmental outcomes
M12	Ranking of 75% of period exceedance values at each location averaged across all locations.	Water supply and environmental outcomes
M13	Ranking of median (50%) number of days for which access to Bushay Campground is limited.	Recreation, power production, and staffing impacts
M14	Ranking of median (50%) annual power production.	Recreation, power production, and staffing impacts
M15	Ranking of mean number of days for which elevation 758.8 ft is exceeded annually.	Recreation, power production, and staffing impacts
M16	Average ranking of the mean number of gate changes by water year from baseline.	Recreation, power production, and staffing impacts

Rank of WCP alternative by flood risk management metrics							
Metric ID	Baseline	EFO	Hybrid	Modified Hybrid	5-day deterministic forecast		
M1	5	1	3	1	4		
M2	3	5	1	1	4		
M4	5	3	2	1	4		
M6	5	4	1	1	3		
M8	1	1	1	1	1		
M9	1	1	1	1	1		
Average	3.3	2.5	1.5	1.0	2.8		

Table 50. Summary of rankings: flood risk management metrics

Table 51. Summary	of ranki	ngs: Water	[,] supply an	d environmental	outcomes	metrics
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Rank of WCP alternative by water supply and environmental metrics							
Metric ID	Baseline	EFO	Hybrid	Modified Hybrid	5-day deterministic forecast		
M10	5	1	4	2	3		
M11	5	1	2	2	2		
M12	5	1	4	2	3		
Average	5.0	1.0	3.3	2.0	2.7		

Table 52. Summary of rankings: Recreation, power production, and staffing impacts

Rank of WCP alternative by recreation, power, dam safety, and operations metrics							
Metric ID	Baseline	EFO	Hybrid	Modified Hybrid	5-day deterministic forecast		
M13	1	5	1	4	3		
M14	4	5	1	1	1		
M15	1	5	2	2	4		
M16	2	1	3	4	5		
Average	2.00	4.00	1.75	2.75	3.25		

Table 53. Summary of rankings: Impacts to Lake Sonoma operations

Rank of WCP alternative by Lake Sonoma flood risk management metrics							
Metric ID	Baseline	EFO	5-day deterministic forecast				
M3	1	1	1	1	1		
M5	1	1	1	1	1		
M7	1	1	1	1	1		
Average	1.0	1.0	1.0	1.0	1.0		



References

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